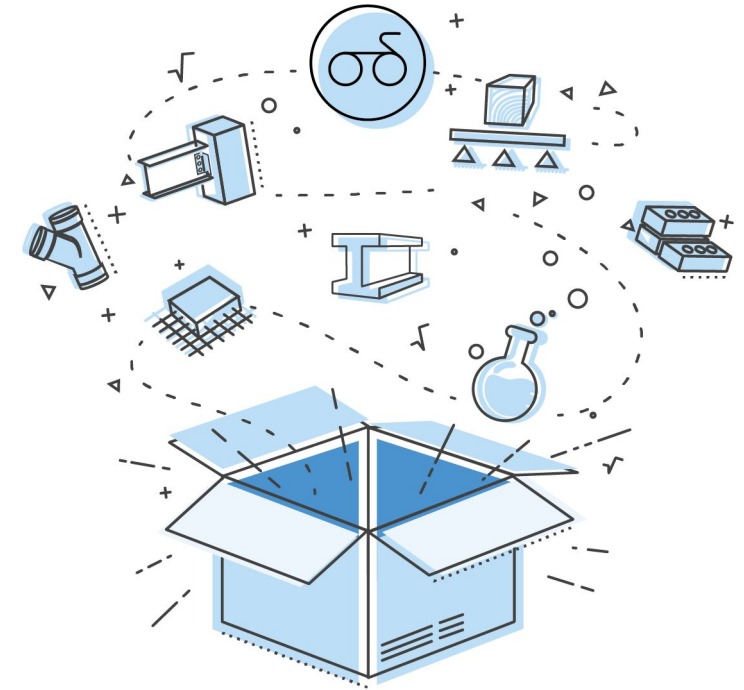


# The Direct Strength Method

## In Cold-Formed Steel Design

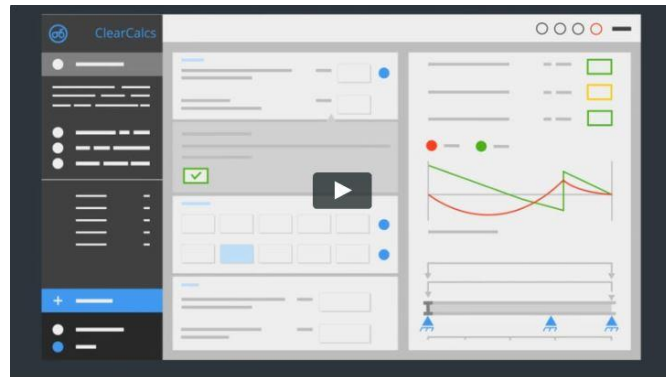


**Brooks H. Smith, CPEng, P.E.**  
brooks.smith@clearcalcs.com

# About ClearCalcs.com

ClearCalcs helps engineers design without compromise by bringing together powerful FEA analysis with easy to use design tools for concrete, steel, cold-formed steel and timber.

Explore our range at [clearcalcs.com](https://clearcalcs.com)



[Intro Video](#)  
[Hyperlink](#)



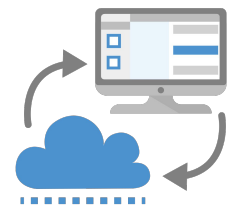
## More Accurate

Design more accurately with unrestricted and accessible FEA analysis



## Eliminates Wasted Time

Eliminate time wasted using clunky methods or waiting for software licenses to free up



## Available Everywhere

Empower engineers to work effectively from office, home, or site

# Meet the Presenter

- **Brooks H. Smith | Head of Engineering R&D**

- Chartered Professional Engineer (AU) & P.E. (USA)
  - MCivE from University of Massachusetts
  - BEng from Dartmouth College
- 8 years of previous experience in:
  - Structural engineering R&D consulting, specialising in cold-formed steel
  - Research fellowship in system behaviour of thin-walled steel
  - Forensic structural engineering, specialising in reinforced and PT concrete
- Almost 5 years now with ClearCalcs
  - Focusing on R&D and QA



# How to Ask Questions

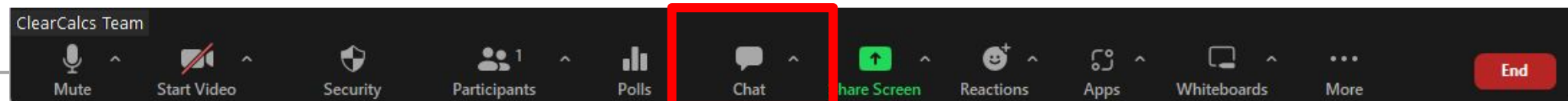
- **Type your questions in the Chat tab on your Zoom control panel and click Send**
  - Please send your questions to everyone
  - We will address all questions in the second half of the webinar during the 15-minute Q&A session
  - We might invite you to unmute yourself to ask your question live!



*Ask your  
questions here*



*You can send to everyone or  
directly to Connor*



# Agenda – Today's Goals

- **Theory on the Direct Strength Method**

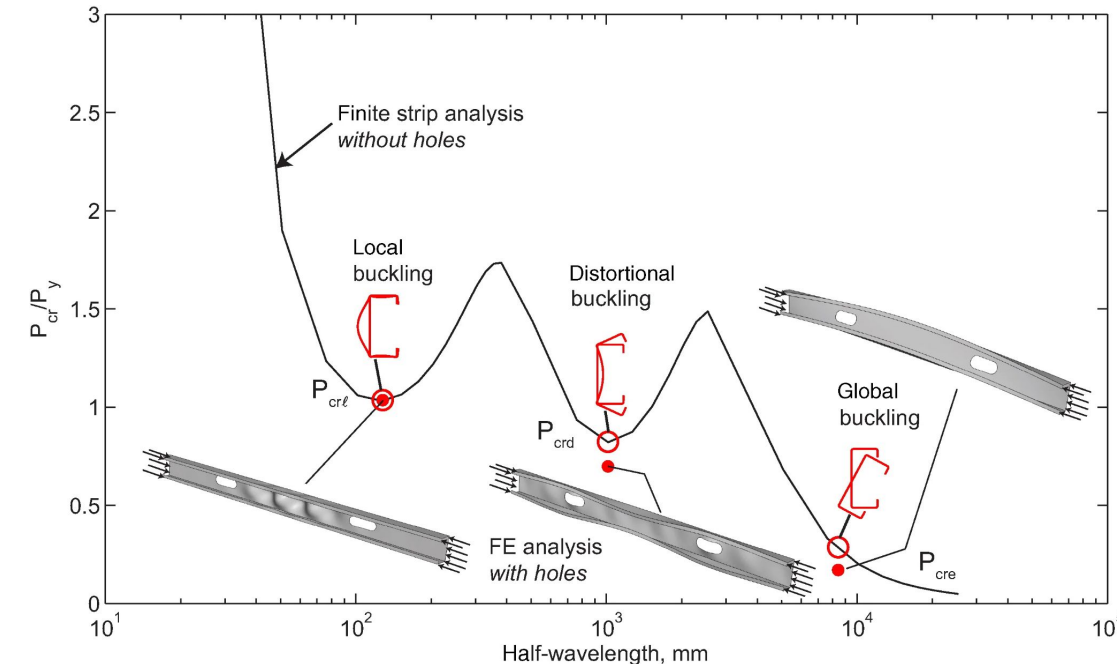
- DSM vs Effective Width Method
- Finite Strip Method

- **Practically Using DSM**

- Keeping Finite Strip Method Simple
- Using DSM in AS/NZS 4600:2018

- **Demonstration in ClearCalcs**

- Just the Design
- Doing a Finite Strip Method Analysis

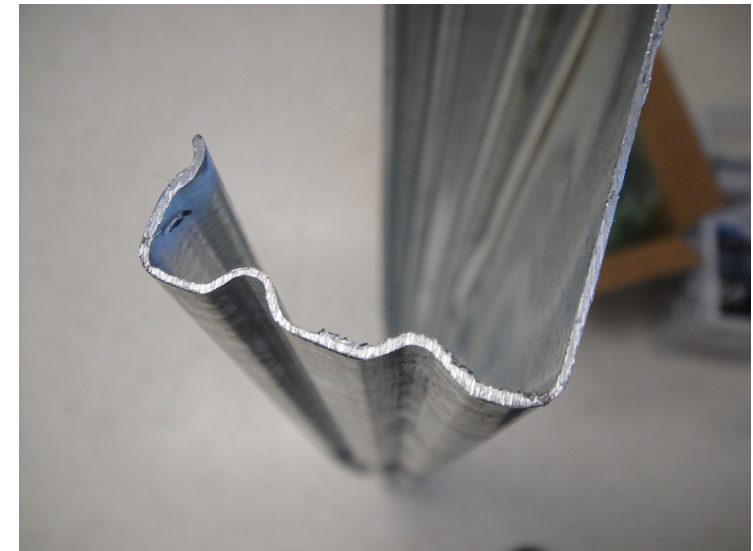
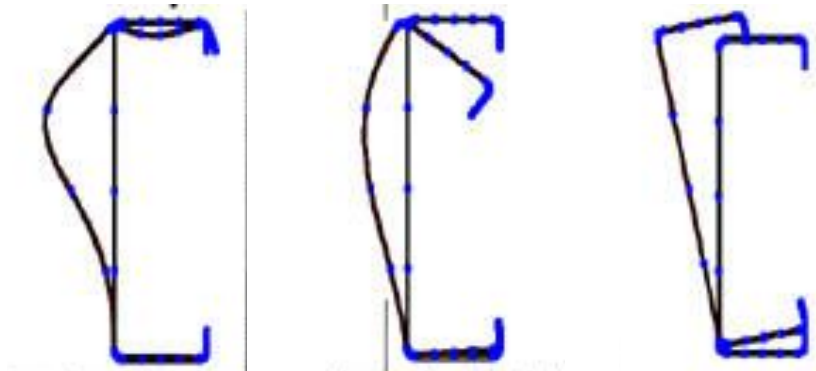


# Theory on the Direct Strength Method

Comparing with the Effective Width Method, and doing a finite strip analysis

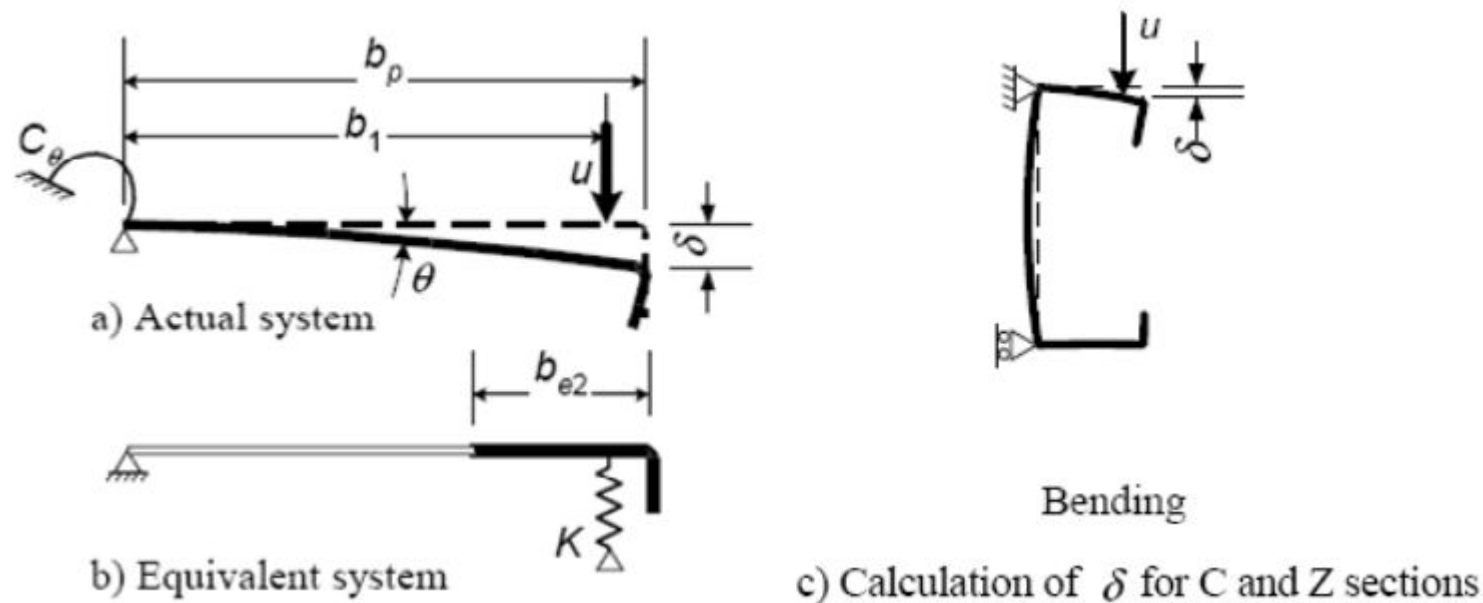
# Buckling in Cold-Formed Steel

- Hot-rolled steel classifies sections as compact, non-compact, or slender – and requires extra equations for “slender”
  - In cold-formed steel, “slender” checks basically always need to be done
- Local, distortional, or global buckling for bending or compression
  - Global encompasses both lateral and torsional buckling
- Stiffeners function to mitigate buckling



# Old Method - Effective Width

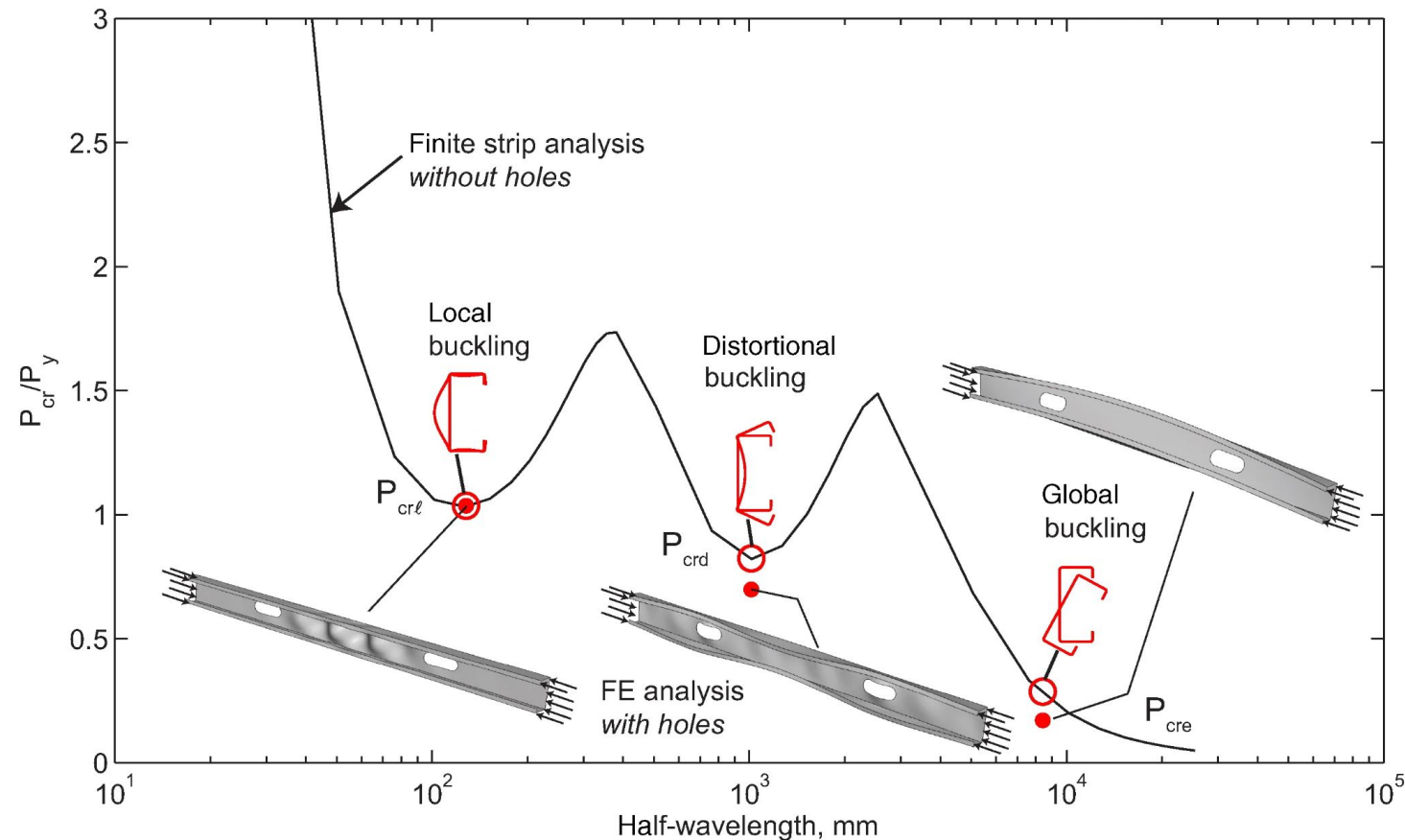
- At its most basic, accounts for buckling by pretending that each 'plate' within an element is shorter than it really is
  - More susceptibility to buckling = shorter 'effective width'
- Separate equations for local vs distortional vs global buckling





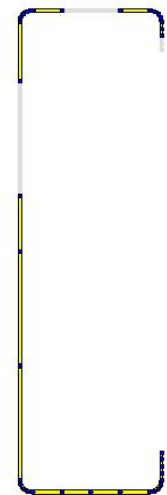
# New Method - Direct Strength

- Directly consider the strength and stiffness of the cross-section *as a whole*
- Consider every half-wavelength for buckling susceptibility
- Every type of buckling in one unified method

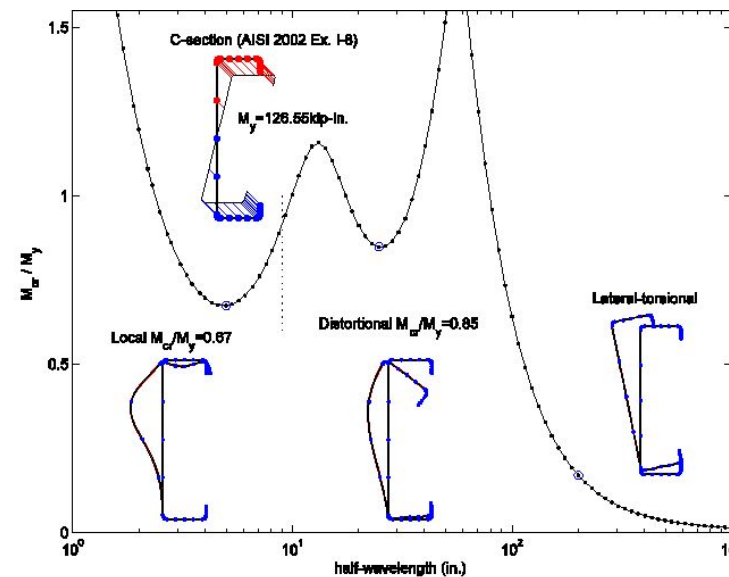


# EWM vs DSM: Why Switch?

- More flexible: EWM is a PIA for complex sections/many plates
  - But every section is exactly as much work for you in DSM!
- More accurate: DSM is will give you up to ~10% more capacity
- Easier calculations: Maths mostly done by finite strip analysis



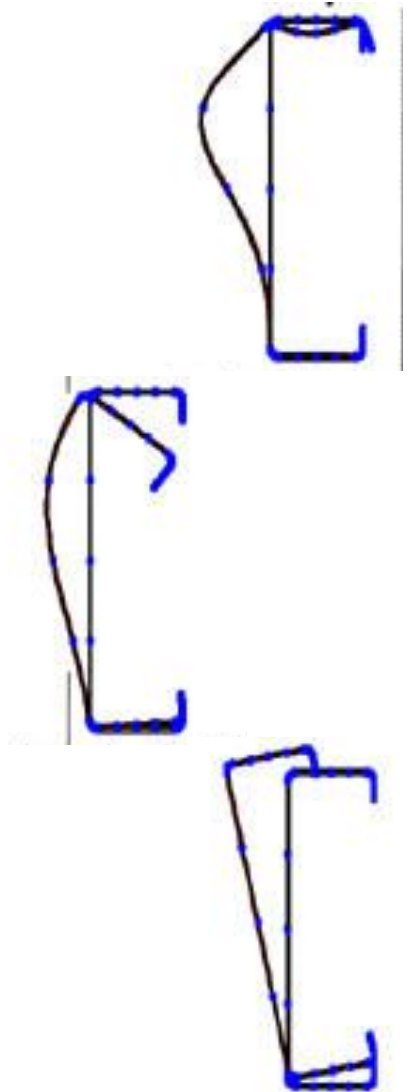
effective section of  
-section in bending



(b) semi-analytical finite strip solution of a C-section in bending showing local, distortional and lateral-torsional buckling

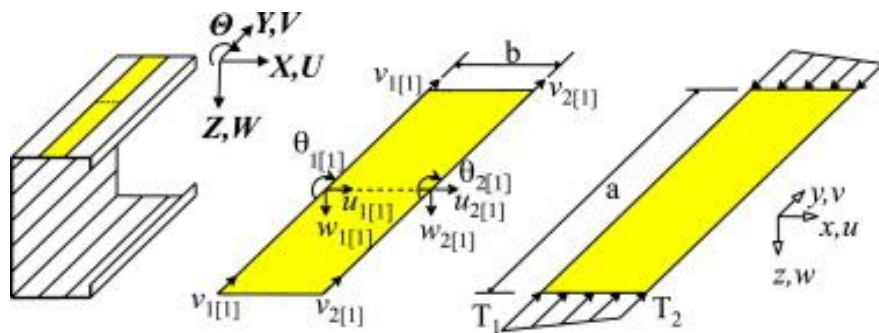
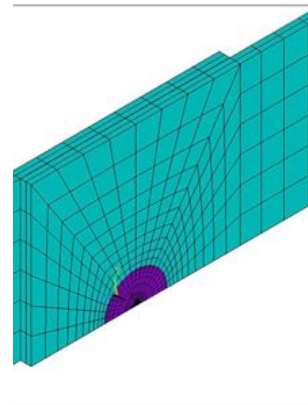
# Interjection: Buckling Modes

- **Local Buckling:** corners of the cross-section stay still, while the flat plates bend
  - Usually occurs at a half-wavelength of about 100-250 mm
- **Distortional Buckling:** corners of the cross-section move, but not all corners move together
  - Usually occurs at a half-wavelength of about 400-800 mm
- **Global Buckling:** whole cross-section rotates or translates as a single unit
  - Usually occurs at a half-wavelength greater than  $\sim 1.5$  m

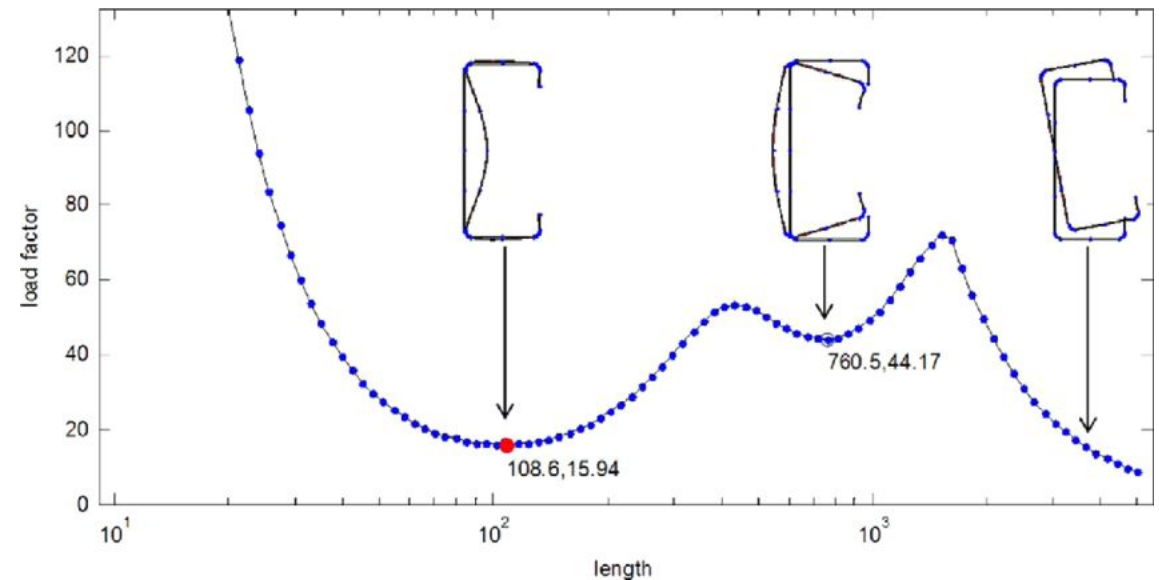


# What is the Finite Strip Method?

- The Direct Strength Method requires a rational analysis that usually takes the form of the Finite Strip Method
  - Instead of finite elements of little triangles or rectangles, we have entire *strips* of arbitrary length
  - All buckling modes



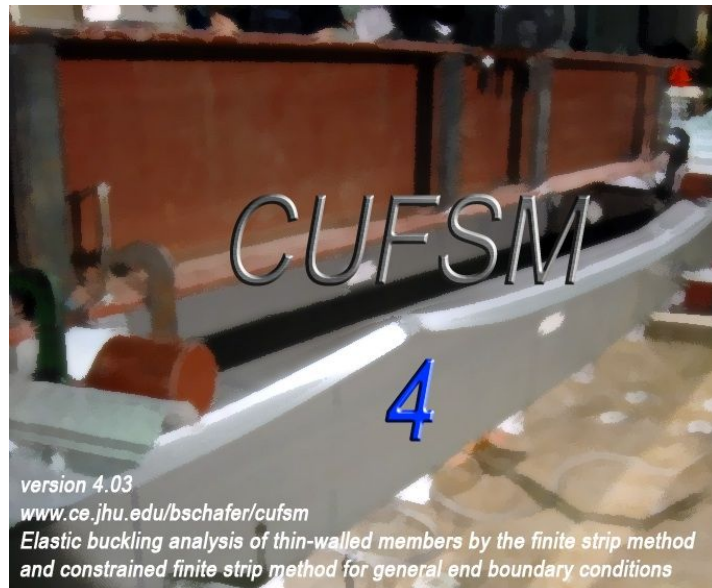
<https://doi.org/10.1016/j.tws.2013.09.004>



<https://dx.doi.org/10.1016/j.tws.2014.01.005>

# Finite Strip Software

- A couple main pieces of software available:
  - [CUFSM](#) (free), from Johns Hopkins University
    - [pyCUFSM](#) (free), ported to Python by Brooks Smith
  - [THIN-WALL](#) (paid), from the University of Sydney



**Centre for Advanced Structural  
Engineering  
THIN-WALL**

# Finite Strip - Signature Curves

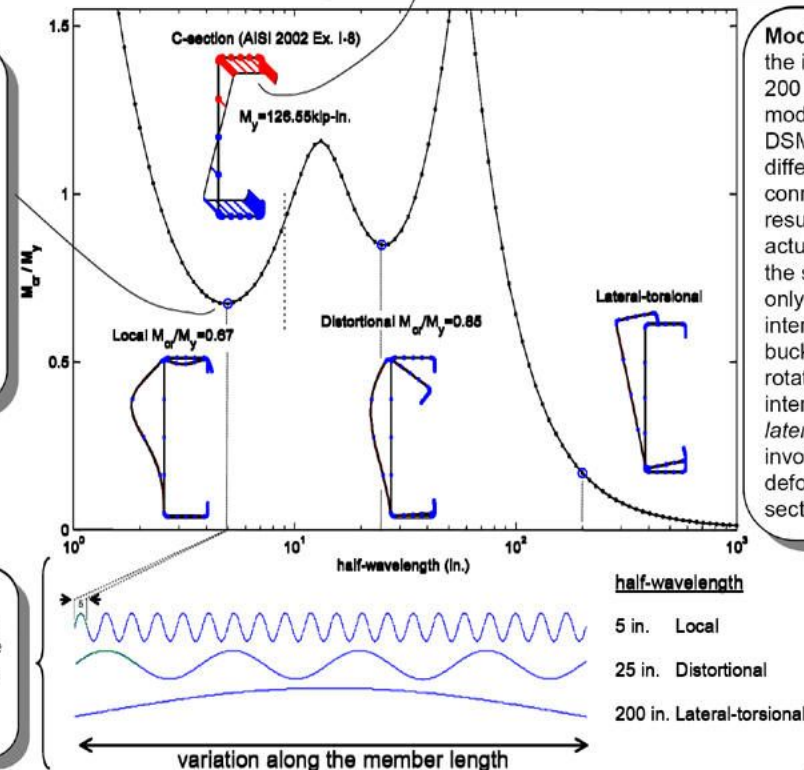
- Commentary to the US's [AISI S100 standard](#) is very helpful (and free!)
  - AS/NZS 4600's DSM section is almost verbatim copied from AISI S100
  - It's just in Imperial instead of Metric units!

## Understanding Finite Strip Analysis Results

Applied stress on the section indicates that a moment about the major axis is applied to this section. All results are given in reference to this applied stress distribution. Any axial stresses (due to bending, axial load, warping torsional stresses, or any combination thereof) may be considered in the analysis.

Minima indicate the lowest load level at which a particular mode of buckling occurs. The lowest  $M_{cr}/M_y$  is sought for each type of buckling. An identified cross-section mode shape can repeat along the physical length of the member.

Half-wavelength shows how a given cross-section mode shape (as shown in the figure) varies along its length.



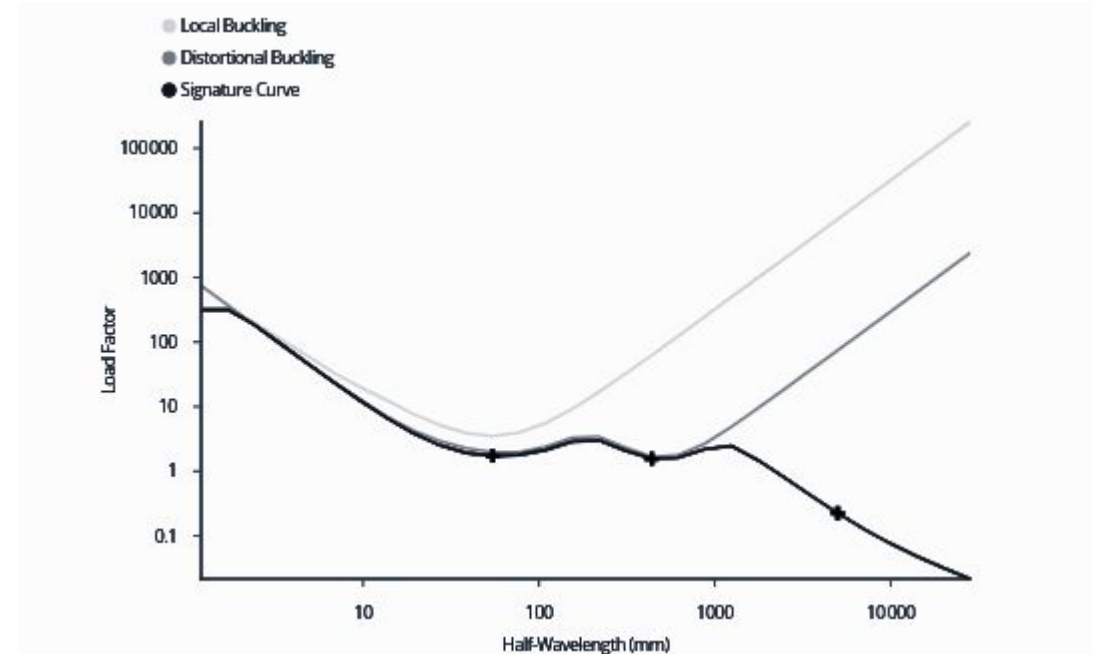
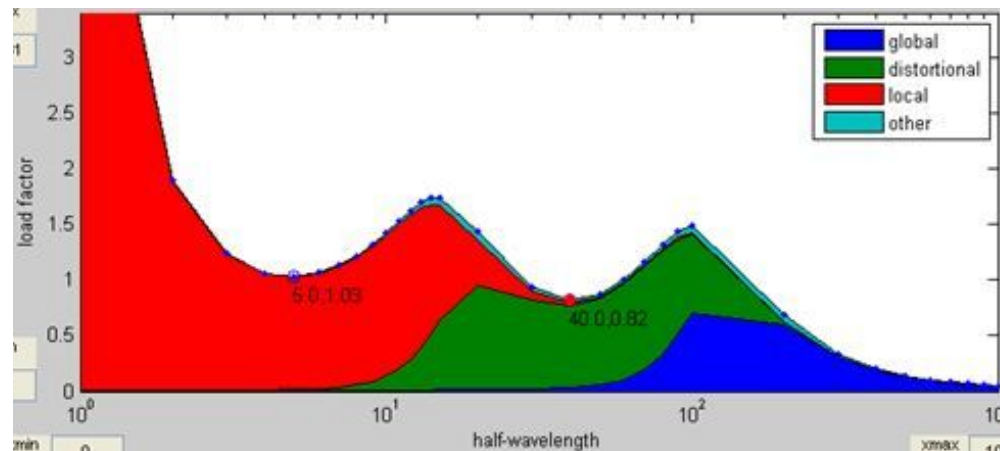
Mode shapes are shown at the identified minima and at 200 in.. Identification of the mode shapes is critical to DSM, as each shape uses a different strength curve to connect the elastic buckling results shown here to the actual ultimate strength. In the section, *local* buckling only involves rotation at internal folds, *distortional* buckling involves both rotation and translation of internal fold lines, and *lateral-torsional* buckling involves "rigid-body" deformation of the cross-section without distortion.

Figure 2 Understanding Finite Strip Analysis Results



# Finite Strip - Mode Classification

- Signature curves are not always as clear as previous example
- “Constrained Finite Strip Method” (cFSM); two methods:
  1. Classifying a signature curve, or
  2. Multiple FSM analyses, with ‘constrained’ modes



# Practically Using DSM

Finite Strip Analysis in real life, and how it fits with AS/NZS 4600:2018



# Direct Strength Method Req'ts (Cl 7.1.2)

- DSM is applicable to *most* sections you may encounter
  - But should still check this:

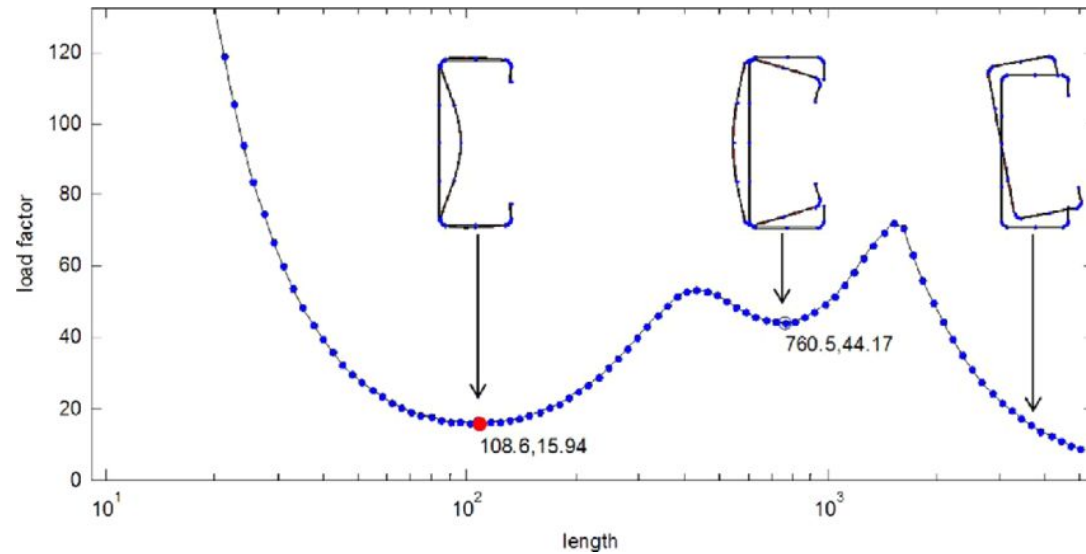


TABLE 7.1  
LIMITS OF APPLICABILITY FOR DESIGN USING  
THE DIRECT STRENGTH METHOD

Criteria	Limiting variables	DSM prequalification limits
Stiffened element in compression [Figure 1.3(C)]	$b_2/t$	$\leq 500$
Edge stiffened element in compression [Figure 2.4.2(a)]	$b/t$	$\leq 160$
Unstiffened element in compression [Figure 2.3.1(a)]	$d/t$	$\leq 60$
Stiffened element in bending [Figure 2.2.3(a)]	$b/t$	$\leq 200$ for unstiffened web $\leq 260$ for bearing stiffener $\leq 300$ for bearing and intermediate stiffener
Inside bend radius	$r_{min}/t$	$\leq 20$
Simple edge stiffener overall length/overall width ratio	$\frac{(d + r_{min} + t)}{(b + 2r_{min} + 2t)}$	$\leq 0.7$
Maximum number of intermediate stiffeners in $b_2$	$n_t$	4
Maximum number of intermediate stiffeners in $b$	$n_{tc}$	2
Maximum number of intermediate stiffeners in web	$n_w$	4
Yield stress used in design	$f_y$	$\leq 655$ MPa

# Local & Distortional Buckling

- Just a couple simple equations for determining if that buckling mode will occur, and then calculating the actual buckled capacity
  - Similar equations for bending moment or axial compression

$$\lambda_l = \sqrt{\frac{M_{be}}{M_{of}}} \quad \dots 7.2.2.3(3)$$

For  $\lambda_l \leq 0.776$ :  $M_{bl} = M_{be}$  ... 7.2.2.3(1)

For  $\lambda_l > 0.776$ :  $M_{bl} = \left[ 1 - 0.15 \left( \frac{M_{of}}{M_{be}} \right)^{0.4} \right] \left( \frac{M_{of}}{M_{be}} \right)^{0.4} M_{be}$  ... 7.2.2.3(2)

$$\lambda_d = \sqrt{\frac{M_y}{M_{od}}} \quad \dots 7.2.2.4(3)$$

For  $\lambda_d \leq 0.673$ :  $M_{bd} = M_y$  ... 7.2.2.4(1)

For  $\lambda_d > 0.673$ :  $M_{bd} = \left[ 1 - 0.22 \left( \frac{M_{od}}{M_y} \right)^{0.5} \right] \left( \frac{M_{od}}{M_y} \right)^{0.5} M_y$  ... 7.2.2.4(2)

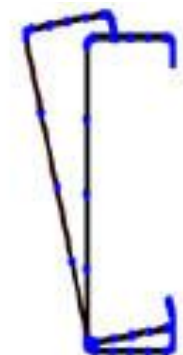
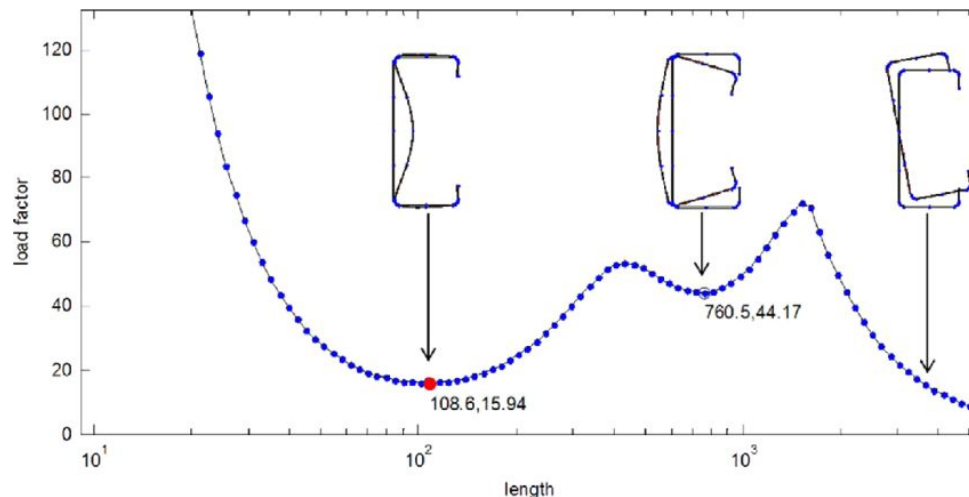


# Global Buckling

- Critical buckling factor can be from Finite Strip Method results
- But, for standard sections, analytical formulae can be easier
  - Just need formulae for  $f_{oy}$ ,  $f_{oz}$ , and (for axial)  $f_{oyz}$

$$M_o = C_b A_g r_{ol} \sqrt{f_{oy} f_{oz}} \quad \dots \text{D2.1.1(1)}$$

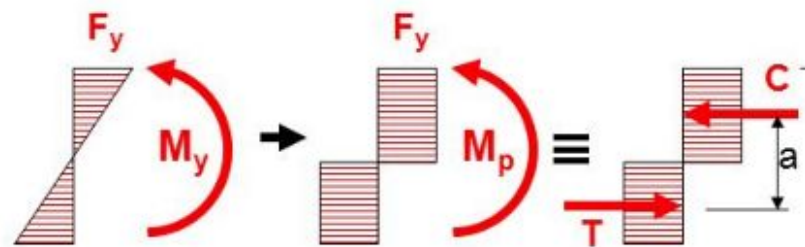
- Then similar equations as local and distortional buckling



# Inelastic Reserve Capacity

- Allows small amounts of localised yielding that don't affect stability
  - Optional; certain connections (like welds) may forbid it
  - Only allowed if critical global buckling  $\gg$  yield strength
- Weighted average of  $M_y$  and  $M_p$

$$M_{bc} = M_p - (M_p - M_y) \left[ \frac{\left( \sqrt{\frac{M_y}{M_o}} - 0.23 \right)}{0.37} \right] \leq M_p \quad \dots 7.2.2.2(5)$$



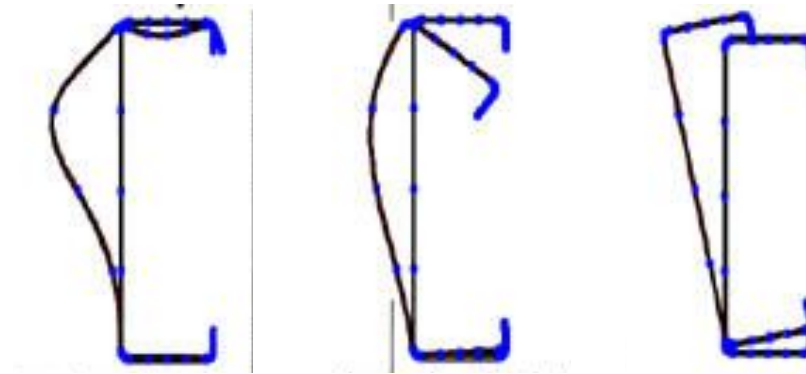
# Final Capacity

- Just simply the minimum of local, distortional, and global buckling capacities

- $\phi_b = 0.9$  in bending, or 0.85 in compression

$$\phi_b M_b = 0.9 * \min(M_{bl}, M_{bd}, M_{be})$$

$$\phi_c N_c = 0.85 * \min(N_{cl}, N_{cd}, N_{ce})$$



# Load Interactions

- **Bending + Shear:** some equations for shear differ slightly
  - We need compatible capacities so that we can combine them

$$\left(\frac{M^*}{\phi_b M_s}\right)^2 + \left(\frac{V^*}{\phi_v V_v}\right)^2 \leq 1.0 \quad 7.2.3(9)$$

- Note that  $M_s$  is a slightly different value - it's  $M_{bl}$  but without consideration of global buckling
  - i.e.  $\lambda_l = \sqrt{M_y/M_{ol}}$  instead of  $\lambda_l = \sqrt{M_{be}/M_{ol}}$

- **Bending + Axial:** first, separately calculate pure bending and pure axial capacities using the Direct Strength Method

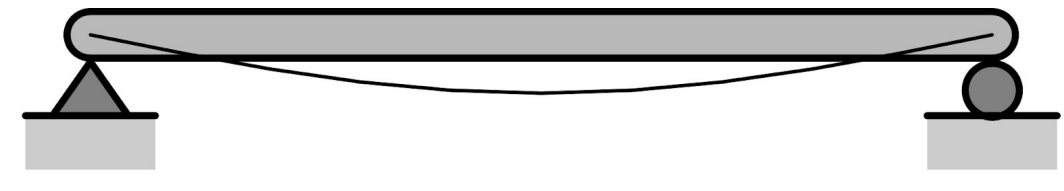
$$\frac{N^*}{\phi_c N_c} + \frac{M_x^*}{\phi_b M_{bx}} + \frac{M_y^*}{\phi_b M_{by}} \leq 1.0 \quad 7.2.4$$

# Deflections

- Also more accurate using our DSM results
  - Conservatively, can use  $I_{eff}$  values given by manufacturers
  - But more accurate is to use our DSM moment capacities
    - $M$  is the maximum service moment demand
    - $M_n = M_b$ , except that we replace all instances of  $M_y$  with  $M$

$$I_{eff} = I_g \left( \frac{M_n}{M} \right) \leq I_g$$

... 7.1.4



# Demonstration in ClearCalcs

How does the workflow look like in ClearCalcs



# Questions?



# THANK YOU!

- We will send you a recording of the webinar by email.
- There will be a survey at the end of this webinar, we would appreciate your feedback on how we can improve.
- If you have further questions, send an email to [help@clearcalcs.com](mailto:help@clearcalcs.com) or use the Help button in ClearCalcs
- Stay tuned for webinar [Webinar Title] next month!

# Appendix

## About ClearCalcs

# Happy Engineers Using ClearCalcs

ClearCalcs has been used in 2,000,000+ designs by a growing number of engineers across the globe, with the US becoming our largest customer base in 2021.



*"You are light years ahead of the competition on features and ongoing growth."*

**Don C.**  
Foundation Engineering Specialists, LLC

*"Why didn't you just use ClearCalcs for that?"*

**Helen W. via Landon R.**  
Criterium Engineers

*"The program basically does the work for you...Wow, I can finally throw away the last of my spreadsheets!"*

**Jason M.**  
J. Michael Engineering, PLLC



# The ClearCalcs Team

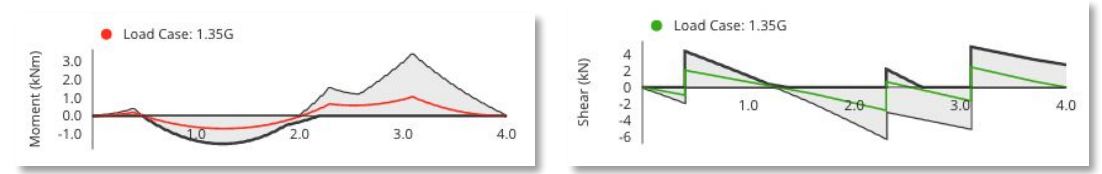
A growing team of passionate engineers, programmers, customer success specialists, product managers, marketers, and more!



# What Sets Our Calculations Apart

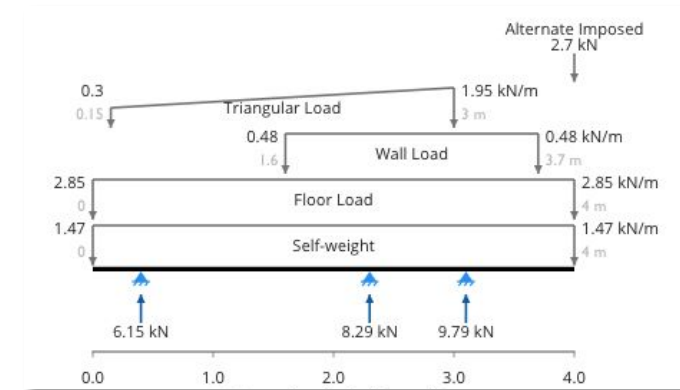
- **Live solutions**

- *Instantly see how every change you make affects the design, in all load cases*



- **Finite Element Analysis**

- *Get the most accurate results no matter what your configuration*



- **As simple or complex as you want**

- *Safely enter in only a few properties, or tune every parameter – it's up to you*

**Key Properties**

Member Type:

Number of Members in Group/Laminate:  $n_{com} =$

Member Orientation:

Total Span Length:  $L =$

---

**Modification Factors (AS1720.1, CI 2.4)**

Initial Moisture Content:  $mc =$

Moisture Content when Fully Loaded:

Equilibrium Moisture Content (Annual Average):  $EMC =$

# What Sets Our Design Process Apart

- **Member selector**

- *Check every possible member in seconds*

Designation	$M_d$	$V_d$	$\delta_l$	$\delta_s$
70 x 35 F5 Seasoned SW	450%	91%	417%	752%
90 x 35 F5 Seasoned SW	273%	71%	198%	354%
120 x 35 F5 Seasoned SW	154%	53%	84%	150%
140 x 35 F5 Seasoned SW	113%	46%	53%	95%
190 x 35 F5 Seasoned SW	62%	34%	22%	38%

- **Link your loads**

- *No need to manually copy reactions into the next sheet – just create a link*

Link to reaction ✕

Roof Lintel RL8

Support	Location (mm)	Governing Reactions $R^*$ (kN)	Permanent Load Reactions $R^*_G$ (kN)	Imposed Load Reactions $R^*_Q$ (kN)
1	0	0.293	0.0667	0.133
2	60	0.293	0.0667	0.133

- **Simple traffic light indicators**

- *See at a glance how close your design is to perfection*


Summary

Moment Demand	$M^* = 2.14$ kNm	
Moment Capacity	$M_d = 2.33$ kNm	92%
Shear Demand	$V^* = 4.29$ kN	
Shear Capacity	$V_d = 9.24$ kN	46%



# What Sets Our Platform Apart

- **Clean, clear printouts**
  - *Beautiful results your clients can understand*
- **See full detail for every field**
  - *References, equations, and more*
- **Rapid product updates**
  - *Receive new features and calculations within days, not years*

	<b>Client:</b>	<b>Date:</b> Oct 17, 2018
	<b>Engineer:</b> Brooks Smith	<b>Job #:</b>
	<b>Project:</b> test	<b>Subject:</b> B7

Summary	
Moment Demand about X-Axis	$M_x^d = 10.3 \text{ kNm}$
Moment Capacity about X-Axis	$\phi M_x = 12.2 \text{ kNm}$ $\phi = M_{x,d} / M_{x,Rd}$
Shear Demand	$V^d = 20.7 \text{ kN}$
Shear Capacity	$\phi V_c = 118 \text{ kN}$ $\phi = V^d / V_c$

Shear Capacity (AS4100-1998, SECTION 5.11)	
Shear Capacity Factor	$\phi = 0.9$
Nominal Shear Yield Capacity	$V_{cy} = 131 \text{ kN}$ $\phi V_{cy} = 118 \text{ kN}$ $\phi V_{cy} = \phi A_s f_y$
Nominal Shear Buckling Capacity	$V_{cb} = 131 \text{ kN}$ $\phi V_{cb} = 118 \text{ kN}$ $\phi V_{cb} = \phi A_s f_y$
Nominal Shear Capacity in Uniform Stress Distribution	$V_c = 131 \text{ kN}$ $\phi V_c = 118 \text{ kN}$
Nominal Shear Capacity	$V_c = 131 \text{ kN}$ $\phi V_c = 118 \text{ kN}$

Moment Section Capacity (AS4100-1998, Cl 5.3)	
Capacity	$M_x = 32.6 \text{ kNm}$ $\phi = 0.9$ $\phi M_x = 29.3 \text{ kNm}$ $\phi M_x = \phi M_{x,Rd}$

Weak Axis Buckling Stress  $f_{oy} = 112 \text{ MPa}$

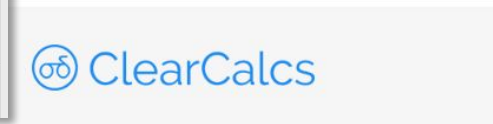
Torsional Buckling Stress  $f_{oz} = 82.2 \text{ MPa}$

Description:  
Buckling stress for torsional global buckling, used to calculate critical elastic buckling stress.

References:  
AS4600-2005, Eqn 3.3.3.2(12)

Conditions:  
(default)  $\rightarrow \frac{GJ}{(A+70^2)} \cdot \left(1 + \frac{\pi^2 E I_{yy}}{(GJ+I_{yy}^2)}\right)$

Flexural-Torsional Factor  $\beta = 0.556$



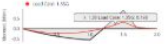
### What's New - Improved connections, diagrams, and more!

New year, stacks of new ClearCalcs updates! We're excited to kick off 2019 with a bang with a bevy of new and imminent updates including new calculation templates and features.

[Log in now](#) and have a look, or read below to find out more.

#### Envelope diagrams

It's now easier than ever to graphically discern the shear, moment, and deflection forces acting on beams, with all diagrams updated to a full





# Key Advantages

ClearCalcs is designed for the modern efficiency focused engineering practice



## More accurate results.

Get far better quality and efficiency than spreadsheets with highly accurate FEM calculations and dynamic load path tracking between members.



## Easy to understand.

Work faster and impress clients and checkers with professional, easy to understand calculations and quick export to PDF.



## Never lose work again.

ClearCalcs was built in the cloud. That means we automatically save your work as you type and keep it securely backed up on our servers.



## Help when you need it.

Need help? Our customer support is built right in to the platform. With a single click you can talk to one of our talented engineers.



## Save time.

Our wide range of templates and easy linking, duplication, and export are all designed to help automate creation of repetitive calculations.



## Always have access.

Shared licenses and lock-outs are a thing of the past! Our simple pricing model makes it easy to give everyone access to ClearCalcs when they need it.



## Easy collaboration.

No more USBs or sending files over email! Everything is shared inside ClearCalcs so you can easily collaborate on projects.



## Upgrades are always free.

We added over 250 updates to ClearCalcs last year. All of our users had them as soon as they were released, and we didn't charge them a cent extra.



## Mobile. Tablet. Desktop.

ClearCalcs was designed to work on any modern device. Nothing to download or install, all you need is a web browser.